# Variation of Vertical Permeability of Fine Soil using Fine Chalk Powder

Saurabh Singh<sup>1</sup>, Babita Pandey<sup>2</sup> and Ashish Sewaliya<sup>3</sup>

<sup>1</sup>Assistant Professor, Department of Civil Engineering, Poornima Institute of Engineering and Technology, Jaipur <sup>2,3</sup>B.Tech. Students, Department of Civil Engineering, Poornima Institute of Engineering and Technology, Jaipur E-mail: <sup>1</sup>saurabh.singh@poornima.org, <sup>2</sup>2015pietcivbabita@poornima.org, <sup>3</sup>2015pietcivashish@poornima.org

Abstract—Soil is base for each development work. Knowing different properties of soil ahead of time like void ratio(VR), liquid limit(LL), plastic limit(PL), plasticity index(PI), permeability (porousness) and so on encourages the architect to develop and outline appropriate and stable structure. In this investigation porousness of soil is recognized and altered utilizing chalk powder containing calcium carbonate for the Jaipur area. Porousness is plot as the property of a permeable material which permits the entry or stream of water (or elective liquids) through its interconnecting voids. Penetrability assumes a critical part in each venture wherever the stream of water through soil might be a worry (e.g. dam drainage, cutoff divider and stomach divider). calcium carbonate is one among the establishing operator and take an interest inside the authoritative of soil particles along through physico-synthetic components and hypothetically makes a steady soil structure. On expanding the measure of chalk powder containing calcium carbonate penetrability of blend diminishes.

Keywords: Permeability, plartic limit, liquid limit.

## 1. INTRODUCTION

As the district changes the properties of soil likewise changes with the area, it is impacted by its physical substance, atmosphere and climate. Amid planning of structure it is critical for specialists to comprehend about the dirt underneath on the grounds that this will influence the dependability of structure. Geotechnical properties incorporate grain measure conveyance, liquid limit(LL), plastic limit(PL), porosity, void ratio(VR), particular gravity, soli arrangement, shear quality and compressibility. In the event that dirt is frail in a few properties than soil adjustment is finished utilizing lime, concrete any more materials to enhance the properties of soil. In current examination chalk powder of particular size is utilized as a balancing out material. Chalk powder does not have dissolving point and breaking point, deteriorates into calcium oxide and carbon dioxide, thickness of chalk is 1.7 g/cm3, can't lead power, shine of chalk is white, dry and can hold a lot of water.

On the results of consolidometer tests. The samples Expansion of lime to a dirt has a tendency to enhance its properties because of soil-lime responses. The benefit of this property

generally for street subgrade has been used adjustment[1],[2].[3] Expanding use is being made of lime adjustment for mass fill activities, banks, and cutting incline repair as an orientation stratum for delicately stacked establishments. This is notwithstanding lime slurry weight infusion, lime segments, and lime heaps for ground change[4],[5]. Two principle distinctive aftereffects of lime activity are normally accepted to happen in soil-lime blends. The principal, brisk activity, happens quickly on expansion of lime and is accepted to be because of cation trade and flocculation. The second response, total, which is timesubordinate (days to years), is somewhat moderate. It is ascribed to the pozzolanic responses with development of cementation [6],[7],[8]. A standout amongst the most critical focuses contemplated in soil mechanics is the impact of water on the geotechnical properties of soils. Porousness has not been considered as widely as the other significant soil building properties like quality and compressibility. In any case, as of late a generous measure of work has been done by different analysts [9],[10] on penetrability.

The aftereffect of lime on soil penetrability is as yet not clear. Fossberg[11] announced an abatement of around two requests of extent in the coefficient of porousness with expanding lime substance, and observed the lessen to be in essential extent to the lime substance and curing time. He based his discoveries for the most part were compacted 1 h in the wake of blending the dirt and lime.[12], then again, reported a considerable increment in the coefficient of porousness of overwhelming mud soils (from 2 X  $10^{-8}$  cmls to 4 X  $10^{-6}$  cm/s).

In current study soil & chalk particles passing through 1.16mm sieve and retained on 150 micron sieve are used. Samples containing different amount of chalk particles as follows 0, 20, 40, 60, 80, 100 percent in soil are created.

Porousness alludes to the straightforwardness with which water can course through a dirt. Inside the setting of soil, porosity commonly identifies with the affinity of a dirt to allow liquid to move through its void regions. This property is basic for the estimation of waste through earth dams or underneath load dividers, the calculation of the spillage rate from misuse storerooms (landfills, lakes, etc), and thusly the tally of the rate of settlement of clayey soil deposits. There are two general sorts of permeability test procedures that are routinely performed in the exploration office: the enduring head test system is used for vulnerable soils ( $k>10^{-4}$  cm/s), the falling head test is generally used for less permeable soils ( $k<10^{-4}$  cm/s).

In current study falling head method is used to find the permeability of soil samples. According to Darcy's law discharge per unit time is directly proportional to hydraulic gradient.

> Q=K\*i\*a K=[2.3 a.L / (A.Δt)].log(h\_U / h\_L)

Where,

q: discharge per unit time

L: height of the soil sample column

A: sample cross section

a: cross section of the standpipe

 $\Delta t$ : recorded time for the water column to flow though the sample

 $h\_U$  and  $h\_L$  : upper and lower water level in the standpipe measured using the same water head reference

# 2. STUDY AREA

The soil sample is collected from Sitapura, Jaipur region. Soil is dried completely before use. The moisture content present in soil was 16.6% before drying the soil.

## 3. MATERIAL USED

Chalk having density  $1.7 \text{ g/cm}^3$ , then crushed into fine powder form and passing through 1.18mm sieve and retained on 150 micron sieve.

Soil is collected from the study area. After drying the soil, sieve analysis of soil is done. Soil passing through 1.16 mm sieve and retained on 150 micron sieve. To check the effect of permeability on soil, varying amounts [0% (100S), 20% (20C80S), 40% (40C60S), 60% (60C40S), 80% (80C20S) and 100% (100C)] of chalk powder samples are prepared.

Each sample of soil mixture were tested for specific gravity based on ASTM D854, Atterberg limit tests based on ASTM D4318, Permeability of the different soil mixtures were determined by the falling head method. The direction of flow of water is also important, thus, a vertical orientation of permeameter is used.

## 4. RESULT AND DISCUSSION

Using ASTM D854 the specific gravity of each soil mix is determined. The obtained resultsof various soil mixtures are shown in Table-1.

With the results shown on Table-1, the addition of chalk powder reduces the specific gravity of a soil mixture. Concentration of voids is increasing on increasing the amount of chalk powder.

Table 1: Specific Gravity of Soil Samples

S. No.	Soil Mixture	Specific gravity
1	0C100S	3.1
2	20C80S	2.94
3	40C60S	2.7
4	60C40S	2.5
5	80C20S	1.9
6	100C0S	1.7

Plasticity index (PI) is the scope of water substance esteems where the dirt shows plastic properties; PI is the distinction between fluid point of confinement and plastic utmost (PI = LL-PL). The Atterberg furthest reaches of soils are vital in development. Pliancy list and fluid point of confinement are viewed as essential in co-relating building properties, as they are affected by indistinguishable elements like mud minerals exist in soil, particles exist in pore water and stress history, i.e. regardless of whether regularly solidified or over united, of soil store. The impact of including chalk powder in the blend versatility record continues expanding appeared in table-2. Pliancy record of soil depends mainly on earth content in soil. So soils that have high versatility record are considered to keep an eye on dirt. With the diminishing in molecule measure, a quick increment in pliancy list is watched. In this way versatility file is a measure of fineness of particles. Pliancy list in connection with fluid point of confinement, give us important data to soil grouping

Table 2: Liquid Limit, Plastic Limit & Plasticity Index of Soil Mix

S. No.	Name	LL	PL	PI
1	0C100S	13	11	2
2	20C80S	15	13	2
3	40C60S	21	15	6
4	60C40S	26	16	10
5	80C20S	29	20	9
6	100C0S	33	22	11

Using falling head permeability method permeability of all mix is identified as shown in table 3. It is observed that permeability of mix is decreasing on increasing the amount chalk powder. Variation of permeability is uniform till 80% addition of chalk powder but after that saturation occurs.

fable 3: Permeat	oility of	Soil	Mixtures
------------------	-----------	------	----------

S. No.	Mix	Permeability
1	0C100S	5.10E-05
2	20C80S	4.30E-05
3	40C60S	3.40E-05
4	60C40S	2.30E-05
5	80C20S	1.00E-05
6	100C0S	9.00E-06



Fig. 1: Permeability of soil mixtures

#### 4.1 Regression Model

To check the impact of chalk powder when added to soil, soilchalk blends, for example, 100S0, 20C80S, 40C60S, 60C40S, 80C20S and 100C are tried. Their porousness esteems are utilized to produce relapse demonstrate. Connection between the level of chalk powder and porousness is set up. The portrayed relapse models are appeared on figure 2. It can be seen that the relapse display take after the pattern that was seen with the trial estimations of the dirt chalk blends, on account of the coupling property of chalk, once it is expanded, the waste is additionally diminished. The lessening in seepage is because of its small scale texture, which is a blend of to a great degree strand grains.

On applying the relapse investigation to the watched information and level of chalk powder information. It has been watched that the estimation of coefficient of assurance R2 is 0.975 upto the 80% measure of chalk powder. The coefficient of assurance, R2, is helpful in light of the fact that it gives the extent of the difference (variance) of watched penetrability. It is a measure that enables us to decide how certain one can be in making forecasts from a specific model/chart. Solid relationship exit between the watched information and level of chalk powder.

The models use the level of chalk powder as the free factor, while the vertical penetrability is the needy factors, The models can anticipate the porousness (vertical arranged) of any dirt chalk blend, once the level of chalk powder is accessible given the grouping of chalk powder is under 80%.

#### 4.2 Validation

To check the Experimental Data versus Regression Model, the deliberate Coefficients of Permeability for each dirt blend are contrasted and the anticipated Coefficient of Permeability of Regression Model. A line that shows fairness between the variable estimated (Experimental Data) on the vertical hub of a chart and the variable anticipated (Regression Model Data) on the level hub The uniformity line diagram is appeared on Figure-3. Watched coefficient of assurance R2 is 0.976 which showed the high precision of model.



Fig. 3: Variation of Observed Permeability with Predicted Pemeability



Journal of Civil Engineering and Environmental Technology p-ISSN: 2349-8404; e-ISSN: 2349-879X; Volume 5, Issue 1; January-March, 2018

### 5. CONCLUSION

Chalk powder is the recommended over other materials to fine sand soil, since cheaper materials are aimed to be utilized. The addition of chalk powder to soils changes the inter-particle void ratio, it decreases the vertical permeability, and thus, the microscopic characteristics and binding nature chalk may contribute to the decrease in permeability. Based on the tests, soil is a combination of larger sand grains and smaller sand grains to form the micro fabric. Soil particles have almost similar size, forming larger inter-particle void, contributing to a much larger inter-particle voids but due to binding nature of calcium carbonate present in chalk powder mix voids are filled with chalk particles. Due to a smaller inter-particle voids, the permeability of pure soil mix varies from 5.10E-05 to 9.00E-06 cm/s. mixture of soil and chalk powder passing through 1.16 mm sieve and retained on 150 micron sieve follows the definite variation of vertical permeability limited to 80% addition of chalk particle in soil.

#### REFERENCES

- [1] Hunter, D. "Lime-induced heave in sulfate-bearing clay soils." *J. Geotech. Engrg.*, ASCE, 114(2), 150-167, 1988.
- [2] Locat, Y., Berube, M. A., and Choquette, M. "Laboratory investigations on the lime stabilization of sensitive clays; shear strength development." *Can. Geotech. J.*, Ottawa, Canada, 27, 294-304, 1990.
- [3] Hammond, A. A. "Manufacture and use of lime and pozzolana cements in Africa." *Lime and other alternative cements*, Neville Hill, Stafford Holmes, and David Mather, eds., Intermediate Technology Publications, London, U.K., 35-46, 1992.

- [4] Rogers, C. D. F., and Bruce, C. J. "Slope stabilization using lime." *Proc. Int. Conf. on Slope Stability Engrg.*, Institution of Civil Engineering, London, U.K.
- [5] Rogers, C. D. F., and Glendinning, S. "Deep-slope stabilization using lime piles." *Transp. Res. Rec.* 1440, Transportation Research Board, Washington, D.C., 63-70., 1994.
- [6] Webb, D. J. T. "Lime stabilized soil blocks for Third World housing." *Lime and other alternative cements*, Neville Hill, Stafford Holmes, and David Mather, eds., Intermediate Technology Publications, London, U.K., 246-257, 1992.
- [7] Rogers, C. D. F., and Lee, S. J. "Drained shear strength of lime clay mixes." *Transp. Res. Rec. 1440*, Transportation Research Board, Washington, D.C., 53-62., 1994.
- [8] Narasimha Rao, S., and Rajasekaran, G. "Reaction products formed in lime-stabilized marine clays." J. Geotech. Engrg., ASCE, 122(5), 329-336, 1996.
- [9] Yin, J., Finno, R. J., Feldkamp, J. R., and Chung, K. Y. "Coefficient of permeability from AC electroosmosis experiments. I: Theory." J. Geotech. Engrg., ASCE, 122(5),346-354, 1996.
- [10] Finno, R. J., Chung, K. Y., Yin, J., and Feldkamp, J. R. "Coefficient of permeability from AC electroosmosis experiments. II: Results." J. Geotech. Engrg., ASCE, 1996, 122(5), 355-364.
- [11] Fossberg, P. E. "Some fundamental engineering properties of lime-stabilized clay." *Hwy. Res. Rec.* 236, Highway Research Board, Washington, D.C., 19-26., 1969
- [12] Townsend, D. L., and Klym, T. W. "Durability of lime stabilized soils." *Hwy. Res. Rec.* 139, Highway Research Board, Washington, D.C.,25-41,1966.